

## City of Napoleon, Ohio

# Zoning Department 255 West Riverview Avenue, P.O. Box 151

Napoleon, OH 43545 Mark B. Spiess, Senior Engineering Technician / Zoning Administrator Telephone: (419) 592-4010 Fax: (419) 599-8393 www.napoleonohio.com

#### **RESIDENTIAL ZONING PERMIT**

Issued Date:

July 5, 2019

**Expiration Date:** 

July 5, 2020

Permit Number:

P-19-173

Job Location:

221 E. Barnes Ave.

Owner:

Grant Douglas Adkins

221 E. Barnes Ave. Napoleon, Ohio 43545

Contractor:

Schuette Construction

419-705-9524

Zone: R-2 Low Density Residential

Set Backs: Accessory Building

Front: 50 Rear: 10 Side: 7

Comments:

Build New Garage on Existing Foundation

Permit Type: Zoning

Fee: \$25.00

Status: Paid

Amount Due: \$0.00

Mark B. Spiess

Sr. Eng. Tech/Zoning Admin.



## City of Napoleon, Ohio

Zoning Department grant. Chevyguy @ gnail. Com
255 West Riverview Avenue, P.O. Box 151

gnail. Com

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Residential Zoning Permit Application

BATCH# 4119 CHECK# 34	DATE	8.26.0019	
SIGNATURE OF APPLICANT Just No. Och	_		DATE: 8/26/19
I HEREBY ACKNOWLEDGE THAT I HAVE READ AND FULLY UNDERSTAND THE AI	BOVE LISTED INSTRUCTIO	NS.	
as his/her authorized agent and I agree to conform to all applicable laws of the jurisdiction. In addi or the code official's authorized representative shall have the authority to enter areas covered by su permit.	ition, if a permit for Work descri ch permit at any reasonable hou	bed in this application is issued, a r to enforce the provisions of the	l certify that the code official code(s) applicable to such
I hereby certify that I am the Owner of the named property, or that the proposed work is authorized	by the Owner of record and that	I have been authorized by the O	wner to make this application
ALTERATION OF ANY BUILDING STRUCTURE, SIGN, OR PART THEREOF AND NO PERMIT APPLIED FOR HEREIN HAS BEEN APPROVED AND ISSUED BY THE CITY OF	USE OF THE ABOVE SHALI	BE UNDERTAKEN OR PER	FORMED UNTIL THE
I FULLY UNDERSTAND THAT NO EXCAVATION, CONSTRUCTION OR STRUCTURAL	L ALTERATION, ELECTRIC	CAL OR MECHANICAL INST	\$ ALLATION OR
respection recoursing the Corporation Limits - Increase 50%		LDG 520.0000.44830) <b>FOTAL FEE:</b>	
Inspection Fee Outside the Corporation Limits - Increase 50%		LDG 520.0000.44830)	\$
Sewer Tap For Lots (Three Fanny) 50,507 Sq. Ft. or Greater - \$550.00 Sewer Tap Inspection Fee For Single Family or Duplex - \$60.00		LDG 520.0000.44830)	\$
Sewer Tap For Lots (Three Family) 36,367 Sq. Ft. or Greater - \$350.00		-	\$
Sewer Tap For Lots (Three Family) 7,201 to 36,366 Sq. Ft. (	00.040	LDG 520.0000.44830)	\$
Sewer Tap For Lots (Two Family) 23,867 Sq. Ft. or Greater - \$200.00		LDG 520.0000.44830)	\$
Sewer Tap For Lots (Two Family) 7,201 to 23,866 Sq. Ft. (	х\$0.012) (МВ	LDG 520.0000.44830)	\$
Sewer Tap For Lots (Single Family) 12,200 Sq. Ft. or Greater - \$60.00		LDG 520.000.44830)	\$
Sewer Tap For Lots (Single Family) 7,201 To 12,199 Sq. Ft. (	20.044	DG 520.0000.44830)	\$
Sewer Tap For Lots 7,200 Sq. Ft. Or Less - \$0.00		DG 520.0000.44830)	\$
5/8" Meter, Copper Setter and Transmitter Without Tap - \$350.00		DG 510.0000.44730)	\$
3/4" Meter, Copper Setter and Transmitter Without Tap - \$440.87		DG 510.0000.44730)	\$
1" Meter, Copper Setter and Transmitter Without Tap - \$525.00		DG 510.0000.44730)	\$
1" Water Tap, 1" Meter, Copper Setter and Transmitter - \$1,400.00 (			\$
1" Water Tap, 5/8" Meter, Copper Setter and Transmitter - \$1,200.00( 1" Water Tap, 3/4" Meter, Copper Setter and Transmitter - \$1,300.00(			\$
Drainage Permit/Outside Water/Sewer Repair - \$0.00  1" Water Tap, 5/8" Meter, Copper Setter and Transmitter - \$1,200.00(		LDG 510.0000.44730)	\$
Driveway/Sidewalk/Curbing/Patio - \$0.00		ON 100.1700.46690)	\$
Accessory Building Under 200 SF (Detached) - \$25.00	(MZ	ON 100.1700.46690)	\$
Fence/Pool/Deck - \$25.00	(MZ	ON 100.1700.46690)	\$
Zoning Permit - \$25.00	(MZ	ON 100.1700.46690)	\$
Demo Permit - \$100.00 - See Separate Form	(MD	EMO 100.1700.46690)	\$
Estimated Completion Date <u>SEPT. 2019</u> Estima	ited Cost <u>130</u>	000,	
Description of Work to be Performed BUILD NEW	GARNOE BN	EXISTING	FOUND
Contractor SCHUETTE CONSTRUCTION Cell Ph	none # 419-705	7-9524	
Owner Address 221 E. BARNES AVE. NA	APOLEON, OI	+ 43545	
Owner GRAMF ADKINS	Telephone #	9-966-2855	
	73772		
Date 7/5/19 Job Location 221 E. Breve	S AVE. NAL	PRIEDN OF 4:	3545

### DE DANSCO ENGINEERING, LLC

P.O. Box 3400 Apollo Beach, FL 33572 Telephone (813) 645-0166 Facsimile (813) 645-9698

The truss drawing(s) attached have been prepared by Dansco Engineering, LLC under my direct supervision and control based on the parameters provided by Stark Truss Inc.

Densing For Schoots Couse

Job: 1903982-05T

2 truss design(s)

67299-W1

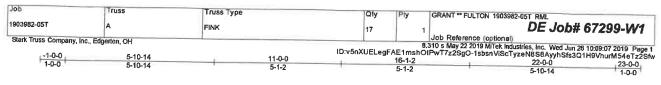
Samuel A. Greenberg, P.E. Ohio Reg. #59715 COA: 02356

Note: Gable end frames with stud lengths exceeding 4' require permanent bracing. On structural gables, where studs may be made from two or more boards as they cross diagonals, the 4' length is the distance from the top chord to bottom chord.

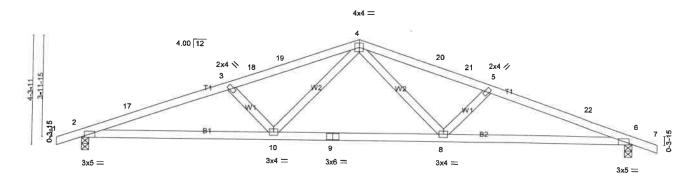
The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI 1-2007 Chapter 2. Further, the attached truss designs comply with the letter and intent of the 2013 Ohio Residential Building Code (ORBC).

Warning !- Verify design parameters and read notes before use.

These designs are based only upon parameters shown, and are for individual building components to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer – not truss designer or truss engineer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 National Design Standard for Metal Plate Connected Wood Truss Construction and BCSI 1-03 Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



Scale = 1:39.3



7-7-4 7-7-4 Plate Offsets (X,Y) [2:0-1-6,Edge] [6:0-1-6,Edge]		14-4-12 6-9-8			22-0-0 7-7-4				
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 19.3/25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           Rep Stress Incr         YES           Code OHIORC13/TPI2007	CSI. TC 0.43 BC 0.58 WB 0.25 Matrix-MSH	DEFL. Vert(LL) Vert(TL) Horz(TL)	in -0.13 -0.31 0.07		I/defl >999 >849 n/a	L/d 360 240 n/e	PLATES MT20 Weight: 71 lb	GRIP 197/144 FT = 20

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 WEBS 2x4 SPF Stud

BRACING-TOP CHORD BOT CHORD

Structural wood sheathing directly applied or 3-9-14 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. (lb/size) 2=1060/0-3-8 (min. 0-1-11), 6=1060/0-3-8 (min. 0-1-11) Max Horz 2=-60(LC 9)
Max Uplift2=-203(LC 10), 6=-203(LC 11)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD

2-17=-2187/345, 3-17=-2132/362, 3-18=-1920/284, 18-19=-1871/290, 4-19=-1860/302, 4-20=-1860/302, 20-21=-1871/291, 5-21=-1920/284, 5-22=-2132/363, 6-22=-2187/346 2-10=-328/2023, 9-10=-141/1384, 8-9=-141/1384, 6-8=-271/2023 BOT CHORD

3-10=-401/194, 4-10=-82/585, 4-8=-82/585, 5-8=-401/194 WEBS

NOTES-

- 1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ff; Cat. II; Exp C; Enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 11-0-0, Exterior(2) 11-0-0 to 14-0-0, Interior(1) 14-0-0 to 23-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 3) TCLL: ASCE 7-05; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=25.0 psf (ground snow); Pf=19.3 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10
  4) Unbalanced snow loads have been considered for this design.

5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 19.3 psf on overhangs non-concurrent with other live loads.

6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads

7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 2=203, 6=203.

LOAD CASE(S) Standard



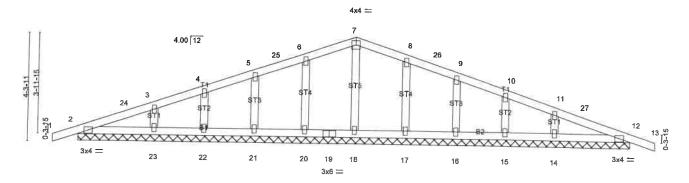
Dansco Engineering, LLC COA: 02356 Date: 6/28/19

WARNING - VERIFY DESIGN PARAMETERS AND READ NOTES BEFORE FABRICATION AND INSTALLATION!!!

This truss design is adequate for the design parameters shown. Review and approval of design parameters is the responsibility of the building designer, not the truss designer or truss engineer. Permanent bracing requirements against out-of-plane buckling are noted/shown for individual truss members (and for the truss as a whole) subjected to gravity and wind loads. Additional permanent bracing design shall be the responsibility of the design professional of record. Temporary and erection bracing shall be the responsibility of the contractor. Reference ANS/TPI-1, "National Design Standard for Metal Plate Connected Wood Truss Construction" and TPI/WTCA BCSI-06, "Building Component Safety Information Guide to Good Practice for Handlina. Installina. Restrainina and Bracino of Metal Plate Connected Wood Trusses" for additional information.

Job	Truss	Truss Type	Qty	Ply	GRANT ** FULTON 1903982-05T RML	
1903982- <b>0</b> 5T	AGE	GABLE	2			# 67299-W1
Stark Truss Company,	inc., Edgerton, OH			1	Job Reference (optional) 8.310 s May 22 2019 MiTek Industries, Inc. Wed Jun Other T772 Sept. 1/2017 2014 Jun 4 1971	26 10:00:00 2010 5
-1-0-0		11-0-0	ID:v5nXUELegi	AE1msh	On will 55 ogo-voor (diammadr Vieiri 80 dB65	2010.09.08 2019 Page 2qWsu_9140qeBvz2Si
1-0-0		11-0-0			22-0-0 11-0-0	23-0-0

Scale = 1:39.3



		22-0-0 22-0-0							40
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 19.3/25.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING-         2-0-0           Plate Grip DOL         1.15           Lumber DOL         1.15           Rep Stress Incr         YES           Code OHIORC13/TPI2007	CSI. TC 0.09 BC 0.05 WB 0.05 Matrix-SH	DEFL. Vert(LL) Vert(TL) Horz(TL)	in 0.00 0.00 0.00	(loc) 12 13 12	l/defl n/r n/r n/a	L/d 180 90 n/a	PLATES MT20 Weight: 74 lb	GRIP 197/144 FT = 2

LUMBER-

TOP CHORD 2x4 SPF No.2 BOT CHORD 2x4 SPF No.2 OTHERS 2x4 SPF Stud

BRACING-

TOP CHORD BOT CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

MiTek recommends that Stabilizers and required cross bracing be installed during truss erection, in accordance with Stabilizer Installation guide.

REACTIONS. ONS. All bearings 22-0-0.
(lb) - Max Horz 2=-60(LC 9)

Max Uplift All uplift 100 lb or less at joint(s) 2, 12, 20, 21, 22, 23, 17, 16, 15, 14

Max Gray All reactions 250 lb or less at joint(s) 2, 12, 18, 20, 21, 22, 17, 16, 15 except 23=256(LC 15),

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

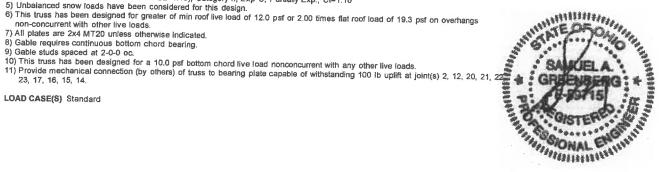
#### NOTES.

- 1) Unbalanced roof live loads have been considered for this design.
  2) Wind: ASCE 7-05; 90mph; TCDL=6.0psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; Enclosed; MWFRS (low-rise) gable end zone and C-C Exterior(2) -1-0-0 to 2-0-0, Interior(1) 2-0-0 to 11-0-0, Exterior(2) 11-0-0 to 14-0-0, Interior(1) 14-0-0 to 23-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- DOL=1.60 piete grip DOL=1.60

  3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.

  4) TCLL: ASCE 7-05; Pr=25.0 psf (roof live load: Lumber DOL=1.15 Plate DOL=1.15); Pg=25.0 psf (ground snow); Pf=19.3 psf (flat roof snow: Lumber DOL=1.15 Plate DOL=1.15); Category II; Exp C; Partially Exp.; Ct=1.10

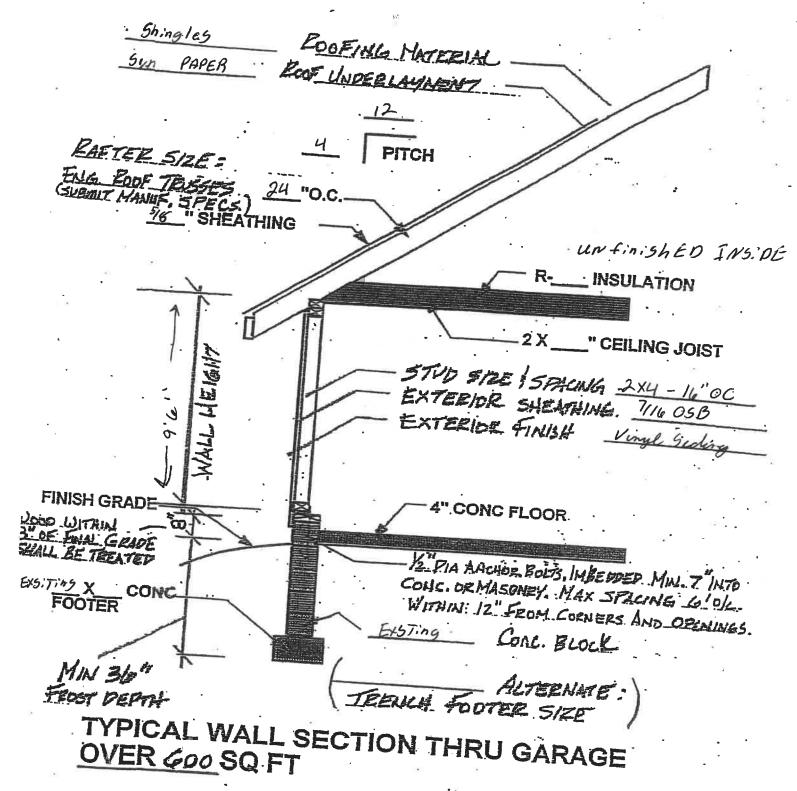
- 5) Unbalanced snow loads have been considered for this design.
  6) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 19.3 psf on overhangs



Dansco Engineering, LLC COA: 02356 Date: 6/28/19

WARNING - VERIFY DESIGN PARAMETERS AND READ NOTES BEFORE FABRICATION AND INSTALLATION!!!

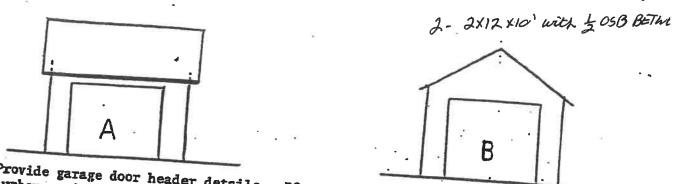
Warning - Veriff Design Parameters and Read Notes before Parameters and installed Inst contractor. Reference ANSI/TPI-1, "National Design Standard for Metal Plate Connected Wood Truss Construction" and TPI/WTCA BCSI-06, "Building Component Safety Information Guide to Good Practice for Handling. Installing. Restraining and Bracing of Metal Plate Connected Wood Trusses" for additional information



1. PROVIDE SIDE HINGED DOOR PER RCO 311. 2.1

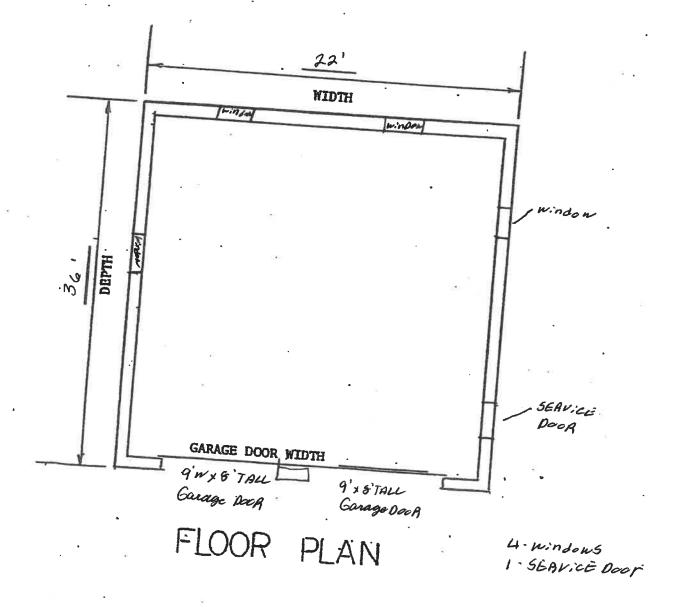
E. REOVIDE HEADER SIZES FOR ALL OPENINGS.

(PROVIDE MANY SPECS. FOR ENGINEERED BEAMS).



NOTE: Provide garage door header details. If garage door is in "A" position, consult lumber yard, engineer, architect or provide engineering data for garage door header.

NOTE: If garage attic is to be used for storage, consult Wood County Code Book for ceiling joist size.



	22 × 36 Garage
	Header 5:2E5
	Garage POOB 2- 2x12x10' & OSB BETWEEN
	SErvice DOOR 2- 2x12x36" " "
	SErvice DOOR 2-2x12x36""""""""""""""""""""""""""""""""""""
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-	
	A